

# PROJECT DESCRIPTION

**SCOPE**  
THE SCOPE OF THIS PROJECT IS TO REPAIR THE EXISTING DAMAGED BRIDGE JOINTS AND EXISTING DAMAGED CONCRETE CURBS TO MATCH THE ORIGINAL DESIGN.

**PROJECT ADDRESS**  
1115 WASHINGTON ST SE  
OLYMPIA, WA 98501

**TAX PARCEL**  
62900500100

**LEGAL DESCRIPTION**  
SECTION 23 TOWNSHIP 18 RANGE 2W QUARTER NW & NE PLAT  
MOORES PD ADDITION TO OLYMPIA DOCUMENT 001/032 571,840 SQ FT  
ALL OF PLAT & VAC STREETS ADJ. EX PTN 14TH AVE  
(JEFFERSON TO ADAMS) PER

**PEDESTRIAN WALKWAY AREA**  
3000 SF

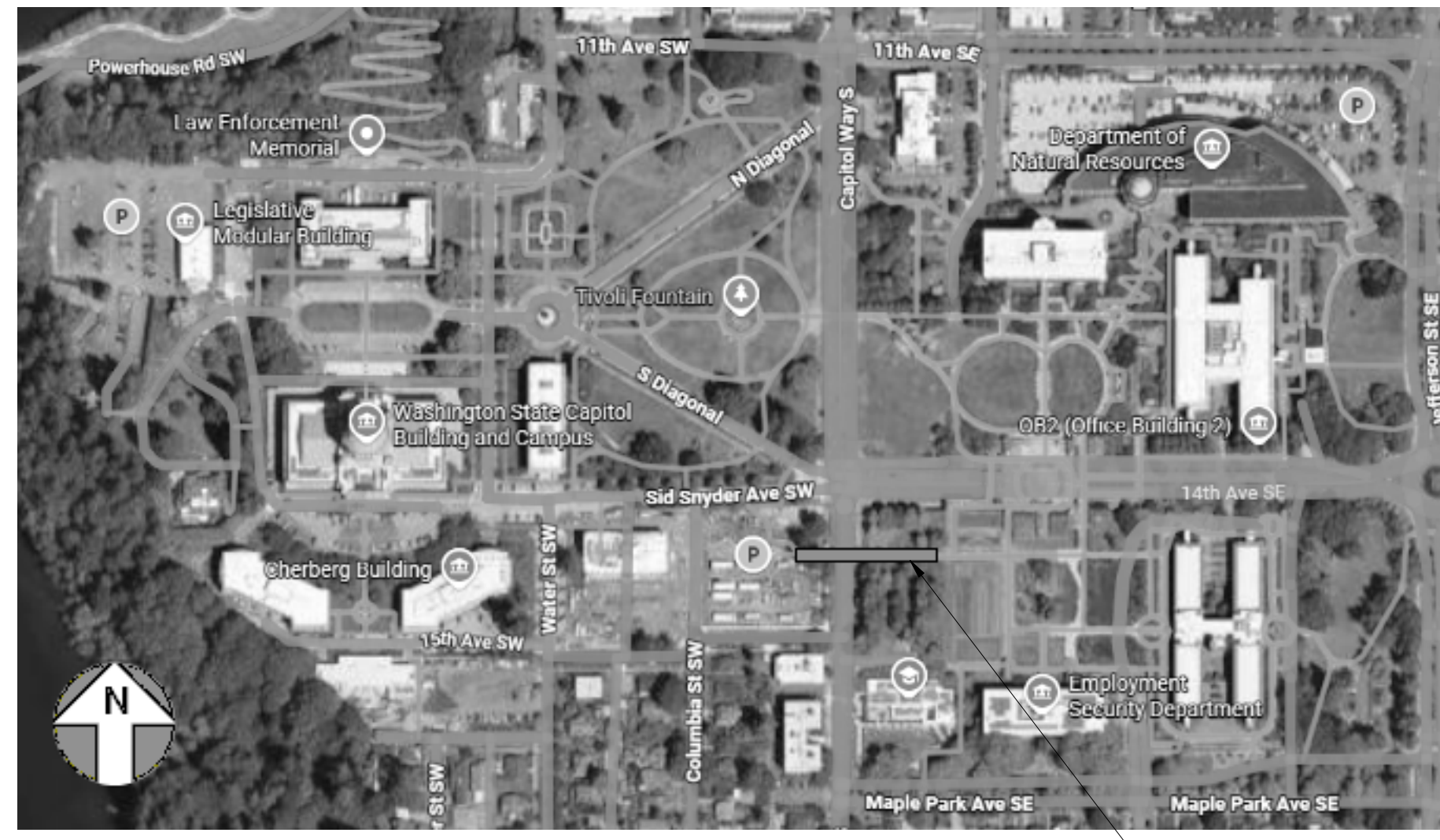
**APPLICABLE CODES**  
INTERNATIONAL BUILDING CODE (IBC), 2021 EDITION, AS AMENDED  
BY THE CITY OF OLYMPIA

# PROJECT DIRECTORY

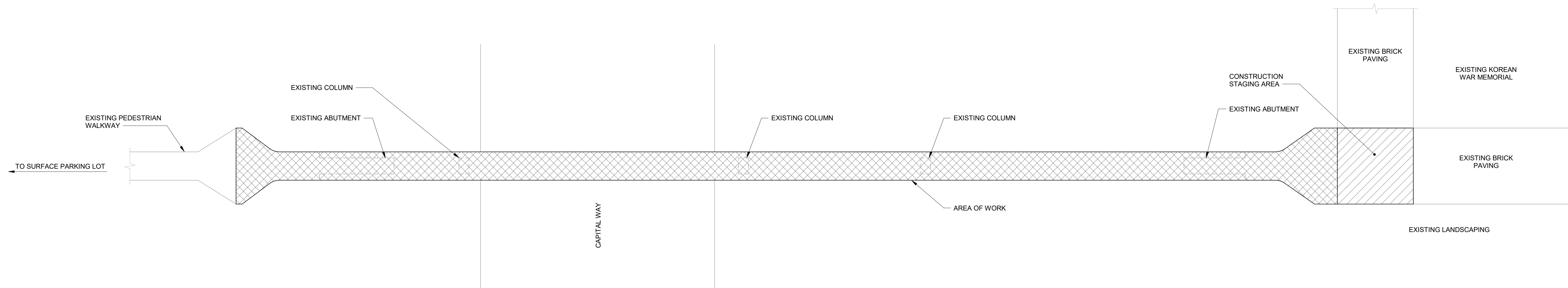
**OWNER**  
EHSAN NIKSAI  
WASHINGTON DEPARTMENT OF ENTERPRISE SERVICES (DES)  
1500 JEFFERSON STREET SE  
OLYMPIA, WA 98501  
360-701-6947  
EHSAN.NIKSAI@DES.WA.GOV

**STRUCTURAL ENGINEER**  
JACOB McCANN  
KPFF CONSULTING ENGINEERS  
1605 FIFTH AVENUE, SUITE 1600  
SEATTLE, WA 98101  
206-622-5822  
JACOB.McCANN@KPFF.COM

# VICINITY MAP



PROJECT SITE



**1 SITE PLAN**  
1/16" = 1'-0"  
PLAN NORTH



SEAL:

CLIENT: DES

## CAPITOL WAY PEDESTRIAN WALKWAY REPAIR

OLYMPIA, WA

WA STATE DES PROJECT NUMBER:  
2026-195 G (1-1)

SUBMITTAL:

ISSUE DATE: --/--

REVISIONS:

MARK DATE DESCRIPTION

SHEET NAME:

## PROJECT INFORMATION

SHEET NUMBER:

# S000

# STRUCTURAL NOTES

## DESIGN LOADS

ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE (IBC), 2021 EDITION, AS AMENDED BY THE CITY OF OLYMPIA.

## GENERAL NOTES

**SUBMITTALS**  
SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO ANY FABRICATION OR CONSTRUCTION FOR ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING: CONCRETE REINFORCEMENT, EMBEDDED STEEL ITEMS, AND STRUCTURAL STEEL.

IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF THE WASHINGTON STATE REGISTERED PROFESSIONAL ENGINEER WHO IS RESPONSIBLE FOR THE DESIGN.

**DEFERRED SUBMITTALS**  
PER IBC SECTION 107.3.4.1, DRAWINGS AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS SHALL BEAR THE SEAL AND SIGNATURE OF THE WASHINGTON STATE REGISTERED PROFESSIONAL ENGINEER WHO IS RESPONSIBLE FOR THE DESIGN AND SHALL BE SUBMITTED TO THE ARCHITECT AND THE BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATION. DEFERRED SUBMITTALS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:

- SHORING OF EXISTING STRUCTURE DURING CONSTRUCTION

**TRAFFIC DIVERSION**  
COORDINATE WITH DES AND THE CITY OF OLYMPIA FOR REQUIRED PERMITS, TRAFFIC, AND PEDESTRIAN DIVERSION FOR ROAD CLOSURES DURING CONSTRUCTION.

## CONCRETE

CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF IBC CHAPTER 19.

**CONCRETE MIXTURES**  
CONCRETE MIXTURES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

CONCRETE MIXTURES						
F <sub>c</sub> (PSI)	TEST AGE (DAYS)	EXPOSURE CLASS			USE	CURBS
		F	S	W		
5,000	28	F3	S1	W2	C2	CURBS

CONCRETE MIXTURES SHALL CONFORM TO THE MOST STRINGENT REQUIREMENTS FOR EXPOSURE CLASSES SPECIFIED IN THE TABLE ABOVE AND ACI 318 TABLE 19.3.2.1.

WATER-REDUCING ADMIXTURES MAY BE INCORPORATED IN CONCRETE MIX DESIGNS, BUT SHALL CONFORM TO ASTM C494, AND BE USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. CA2C2 OR OTHER WATER-SOLUBLE CHLORIDE ADMIXTURES SHALL NOT BE USED.

WATER/CEMENTITIOUS MATERIALS RATIO SHALL BE MEASURED BY WEIGHT AND SHALL BE BASED ON THE TOTAL CEMENTITIOUS MATERIAL. WATER/CEMENTITIOUS MATERIALS RATIO AND WATER CONTENT SHALL BE DETERMINED BY THE SUPPLIER BASED ON STRENGTH REQUIREMENTS AND SHALL NOT EXCEED THE MAXIMUM WATER/CEMENTITIOUS MATERIAL RATIO AND/OR WATER CONTENT IF SHOWN ABOVE OR IN ACI 318 TABLE 19.3.2.1 FOR THE EXPOSURE CLASSES LISTED.

FIELD-MEASURED SLUMP SHALL CONFORM TO THE SUBMITTED CONCRETE MIX DESIGN. TOLERANCE OF SLUMP SHALL CONFORM TO ASTM C94.

ALL CONCRETE SUBJECT TO EXPOSURE CLASSES F1, F2 OR F3 SHALL BE AIR ENTRAINED. AIR-ENTRAINING AGENTS SHALL CONFORM TO ASTM C260. THE PERCENTAGE OF TOTAL AIR SHALL BE ACCORDING TO ACI 318 TABLE 19.3.3.1 WITH A FIELD TOLERANCE OF ±1.5 PERCENT BY VOLUME. THE PERCENTAGE OF TOTAL AIR SHALL BE MEASURED IN THE FIELD AT THE DISCHARGE FROM THE TRUCK.

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR APPROVAL 2 WEEKS PRIOR TO PLACING ANY CONCRETE. THE MIX DESIGN SHALL BE IN CONFORMANCE WITH ACI 318, CHAPTER 19. THE SUBMITTAL SHALL INDICATE WHERE EACH CONCRETE MIX IS TO BE USED ON THE PROJECT, AS WELL AS THE MAXIMUM AGGREGATE SIZE OF EACH MIX. MAXIMUM AGGREGATE SIZE SHALL CONFORM TO THE PROJECT SPECIFICATIONS.

**CURING**  
IF THE AIR TEMPERATURE WILL EXCEED 75 DEGREES F WITHIN 48 HOURS OF PLACING CONCRETE, A MOIST CURE SHALL BE APPLIED TO THE CONCRETE FOR A PERIOD OF 36 HOURS AFTER FINISHING CONCRETE SURFACES. REFER TO THE PROJECT SPECIFICATIONS FOR CURING REQUIREMENTS.

**REINFORCING STEEL**  
• DEFORMED BARS: ASTM A615, GRADE 60, UNO  
• HEADED DEFORMED BARS: ASTM A610, HEAD TYPE HA

REINFORCING SHALL BE SUPPORTED AS SPECIFIED BY THE PROJECT SPECIFICATIONS AND THE CRSI MANUAL OF STANDARD PRACTICE. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI STANDARD OF PRACTICE AS OUTLINED IN ACI 315, "GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS".

MECHANICAL SPLICES CALLED OUT ON THE PLANS SHALL BE TYPE 1, UNLESS OTHERWISE NOTED. TYPE 1 SPLICES SHALL DEVELOP 125 PERCENT OF THE YIELD CAPACITY OF THE SPLICED BARS IN BOTH TENSION AND COMPRESSION. TYPE 2 SPLICES SHALL DEVELOP THE SPECIFIED TENSILE STRENGTH OF THE SPLICED BARS IN TENSION IN ADDITION TO MEETING TYPE 1 SPLICE REQUIREMENTS. SUBMIT ICC-ES OR IAPMO UES REPORT VALID FOR THE 2021 IBC DEMONSTRATING COMPLIANCE OF COUPLERS WITH THESE REQUIREMENTS.

REINFORCING STEEL SHALL HAVE PROTECTION AS FOLLOWS, UNLESS NOTED OTHERWISE:

USE	COVER
• BEAM STIRRUPS:	1 1/2"

WELDING OF REINFORCING, WHERE APPROVED BY THE ARCHITECT, SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES AND PREHEATED IN ACCORDANCE WITH AWS D1.4. REINFORCING STEEL WELDING CODE. WELDERS AND WELDING PROCEDURES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.4. MATERIALS SHALL CONFORM TO THE FOLLOWING:

- REINFORCING BARS TO BE WELDED: ASTM A706, GRADE 60, LOW ALLOY
- WELDING ELECTRODES: E80XX

**NONSHRINK GROUT**  
ALL NONSHRINK GROUT SHALL HAVE MINIMUM F<sub>c</sub> = 5,000 PSI.

**CONCRETE SURFACE REPAIRS**  
REPAIR AND PATCH DEFECTIVE AREAS WHERE NOTED ON DRAWINGS. REMOVE SPALLS AND LOOSE CONCRETE 1/2" TO 3/4" INTO SOLID CONCRETE. MAKE EDGES OF CUTS PERPENDICULAR TO CONCRETE SURFACE. PREPARE SURFACES AS RECOMMENDED BY MORTAR MANUFACTURER. CLEAN, DAMPEN WITH WATER, AND BRUSH COAT EXPOSED SURFACES WITH BONDING AGENT. FILL AND COMPACT WITH PATCHING MORTAR BEFORE BONDING AGENT HAS DRIED.

PATCHING MORTAR SHALL BE SIKATOP-123 PLUS OR APPROVED EQUAL.

## STRUCTURAL STEEL

**REFERENCE SPECIFICATIONS**  
• STRUCTURAL STEEL: AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"

- WELDING: AWS D1.1, TYPICAL  
AWS D1.2 FOR WELDING ALUMINUM
- WELDER CERTIFICATION: AMERICAN WELDING SOCIETY (AWS)  
WASHINGTON ASSOCIATION OF BUILDING OFFICIALS (WABO)

**STEEL MATERIALS**  
• PLATES (PL), BARS: ASTM A36 TYPICAL  
ASTM A572 GRADE 50 WHERE NOTED  
• WELDING ELECTRODES: 70 KSI, LOW HYDROGEN, TYPICAL

STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE REQUIREMENTS OF IBC CHAPTER 22. ALL MEMBERS ARE TO BE ERECTED WITH NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE PLANS. SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE WILL NOT BE ALLOWED WITHOUT PRIOR APPROVAL BY THE ARCHITECT. ALTERNATIVE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL BY THE ARCHITECT.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND UNEQUAL PARTS.

**PROTECTION OF STEEL**  
STRUCTURAL STEEL AND CONNECTIONS, INCLUDING PLATES AND OTHER STEEL ITEMS EMBEDDED IN CONCRETE, WHICH ARE EXPOSED TO WEATHER AND NOT TO BE PAINTED ACCORDING TO THE ARCHITECT, SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. ALL FIELD WELDS ON GALVANIZED MATERIAL SHALL BE COATED WITH BRUSH APPLIED ZINC-RICH PAINT COMPLYING WITH THE SPECIFICATIONS.

ALL COATINGS ARE TO FOLLOW THE SPECIFICATIONS AND PRODUCT MANUFACTURER'S INSTRUCTIONS.

**WELDING**  
ALL WELDING SHALL BE IN CONFORMANCE WITH AISC AND AWS STANDARDS, AND SHALL BE PERFORMED BY AWS-WABO-CERTIFIED WELDERS. ONLY WELDS THAT ARE PREQUALIFIED, AS DEFINED BY AWS, OR QUALIFIED BY TESTING SHALL BE USED. SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS.

WELDS SHOWN ON THE DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON THICKNESS. MINIMUM WELD SIZE SHALL BE 3/16-INCH, UNLESS NOTED OTHERWISE. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD SYMBOLS ARE SHOWN WHERE FIELD WELDS ARE REQUIRED BY THE STRUCTURAL DESIGN. WHERE FIELD WELD IS NOT INDICATED, THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD-WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL ERECTION.

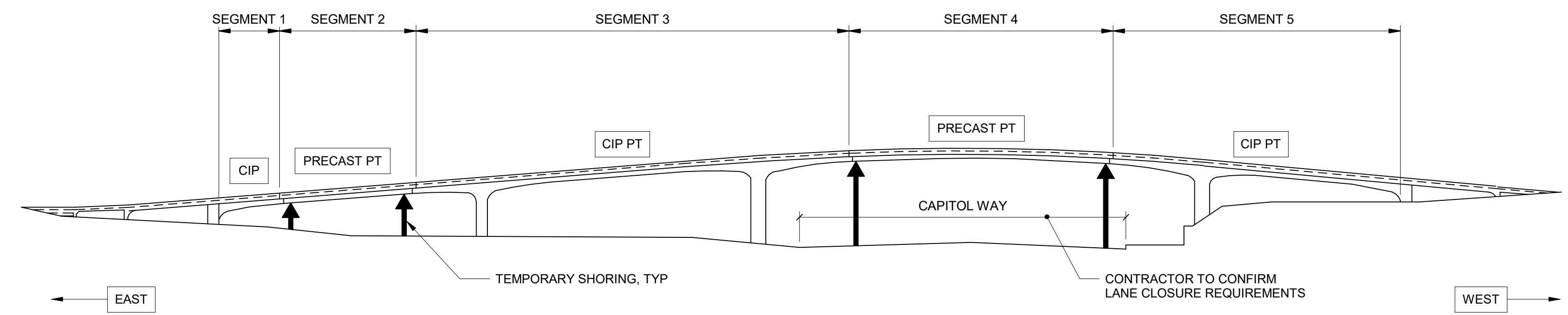
# STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLT	K	KIP (1,000 LB)
ADDL	ADDITIONAL	KSF	KIPS PER SQUARE FOOT
ADH	ADHESIVE	LB	POUND
ADJ	ADJUSTABLE	LF	LINEAR FOOT
ANCH	ANCHOR	LFH	LONG FACE HORIZONTAL
ARCH	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
ARD	ADHESIVE REINFORCING DOWEL	LLV	LONG LEG VERTICAL
BJ	BOTTOM OF	LNGT	LONGITUDINAL
BM	BEAM	LP	LOW POINT
BOT	BOTTOM	MAX	MAXIMUM
BRG	BEARING	MFR	MANUFACTURER
BTWN	BETWEEN	MIN	MINIMUM
CAP	CAPACITY	MISC	MISCELLANEOUS
CC	CENTER TO CENTER	NIC	NOT IN CONTRACT
CJP	CAST-IN-PLACE	NOM	NOMINAL
CJ	CONSTRUCTION OR CONTROL JOINT	NS	NEAR SIDE
CJP	COMPLETE JOINT PENETRATION	NS	NONSHRINK
CL	CENTERLINE	NTS	NOT TO SCALE
CLR	CLEAR	OC	ON CENTER
COL	COLUMN	OD	OUTSIDE DIAMETER
CONC	CONCRETE	OF	OUTSIDE FACE
CONN	CONNECTION	OPNG	OPENING
CONST	CONSTRUCTION	OPP	OPPOSITE
CONT	CONTINUOUS	P	POST
CONTR	CONTRACTOR	PC	PIECE
CONTY	CONTINUITY	PEN	PENETRATION
COORD	COORDINATE	PJP	PARTIAL JOINT PENETRATION
CTR	CENTER	PL	PLATE
CY	CUBIC YARD	PSF	POUNDS PER SQUARE FOOT
DBA	DEFORMED BAR ANCHOR	PSI	POUNDS PER SQUARE INCH
DBL	DOUBLE	PT	POST-TENSIONED
DEMO	DEMOLISH	R	RADIUS
DET	DETAIL	REINF	REINFORCING
DIA	DIAMETER	REQD	REQUIRED
DIAG	DIAGONAL	RND	ROUND
DN	DOWN	RO	ROUGH OPENING
DWG	DRAWING	RTN	RETURN
DWL	DOWEL	SCHED	SCHEDULE
EA	EACH	SECT	SECTION
EF	EACH FACE	SHT	SHEET
EL	ELEVATION	SIM	SIMILAR
ELECT	ELECTRICAL	SPEC	SPECIFICATION
EQ	EQUAL	SQ	SQUARE
ES	EACH SIDE	SST	STAINLESS STEEL
EW	EACH WAY	STD	STANDARD
EX	EXISTING	STIFF	STIFFENER
EXP	EXPANSION	STIRR	STIRRUP
EXT	EXTERIOR	STL	STEEL
FDN	FOUNDATION	STRUCT	STRUCTURAL
FS	FAR SIDE	SUPP	SUPPORT
FT	FEET	SYM	SYMMETRICAL
FTG	FOOTING	T&B	TOP AND BOTTOM
GALV	GALVANIZED	T/	TOP OF
GEN	GENERAL	THK	THICK(NESS)
GOVT	GOVERNMENT	THRU	THROUGH
GR	GRADE	TRANS	TRANSVERSE
HGR	HANGER	TYP	TYPICAL
HK	HOOK	UNO	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL	UT	ULTRASONIC TESTING
HP	HIGH POINT	VERT	VERTICAL
IBC	INTERNATIONAL BUILDING CODE	VIF	VERIFY IN FIELD
ID	INSIDE DIAMETER	W/	WITH
IF	INSIDE FACE	W/O	WITHOUT
IN	INCH	WHS	WELDED HEADED STUD
INFO	INFORMATION	WP	WORK POINT
JT	JOINT	WSBC	WASHINGTON STATE BUILDING CODE



SEAL:

# CONSTRUCTION SEQUENCING



## ELEVATION

- CONSTRUCTION SEQUENCE FOR THIS PEDESTRIAN WALKWAY REPAIR SHALL BE AS FOLLOWS:**
- WHILE PEDESTRIAN BRIDGE IS CLOSED, PROVIDE PEDESTRIAN REDIRECTION PER DES DIVISION PLAN.
  - DEMO EXISTING RAILING AT EACH END OF SEGMENT 2 AND 4 PER DETAIL 9/S201 OR 13/S201.
  - REMOVING EXISTING SURFACE MOUNTED LIGHTS AND CONDUITS AT OP EXISTING CURBS. STORE LIGHTS FOR REINSTALLATION AFTER PEDESTRIAN WALKWAY REPAIR IS COMPLETE.
  - PROVIDE TEMPORARY SHORING AT EACH END OF SEGMENT 2. TEMPORARY SHORING SHALL SUPPORT THE WEIGHT OF THE EXISTING PRECAST PT SEGMENT AND ANY CONSTRUCTION LIVE LOAD ATOP THIS SEGMENT. PRECAST SELF WEIGHT IS 2000 PLF.
  - RAISE UP SEGMENT 2, REPAIR SEGMENT 2 AS NOTED ON S101, AND LOWER SEGMENT 2 BACK TO ITS ORIGINAL LOCATION. SEGMENT 2 TEMPORARY SHORING TO REMAIN IN PLACE.
  - PROVIDE TEMPORARY SHORING AT EACH END OF SEGMENT 4. TEMPORARY SHORING SHALL SUPPORT THE WEIGHT OF THE EXISTING PRECAST PT SEGMENT AND ANY CONSTRUCTION LIVE LOAD ATOP THIS SEGMENT.
  - RAISE UP SEGMENT 4, REPAIR SEGMENT 4 AS NOTED ON S101, AND LOWER SEGMENT 4 BACK TO ITS ORIGINAL LOCATION.
  - REMOVE TEMPORARY SHORING FROM SEGMENT 2 AND SEGMENT 4.
  - RETROFIT EXISTING CONCRETE CURB AND REINSTALL EXISTING SURFACE MOUNTED LIGHTS AND CONDUITS PER 14/S201.
  - RETROFIT RAILINGS AS REQUIRED PER 9/S201 AND 13/S201.
  - AT TOP OF WALKWAY, CLEAN EXPOSED REBAR PER SSPC-SP 3, PATCH WITH GROUT, AND APPLY METHACRYLATE (MMA) DECK SEALER AS A BID ALTERNATE, INSTALL ALTERNATE 1 OR ALTERNATE 2 PER MANUFACTURER RECOMMENDATIONS
    - ALTERNATE 1: SIKELASTIC 2500 MULTI-COAT SYSTEM
    - ALTERNATE 2: SIKELASTIC 720 ONE SHOT SYSTEM
  - CLEAN CONCRETE AT UNDERSIDE AND SIDES OF PEDESTRIAN WALKWAY TO REMOVE EXISTING STAINING FROM PREVIOUS WATER DAMAGE.

CLIENT: DES

## CAPITOL WAY PEDESTRIAN WALKWAY REPAIR

OLYMPIA, WA

WA STATE DES PROJECT NUMBER:  
2026-195 G (1-1)

SUBMITTAL:

-

ISSUE DATE: --/--

REVISIONS:

MARK DATE DESCRIPTION

SHEET NAME:

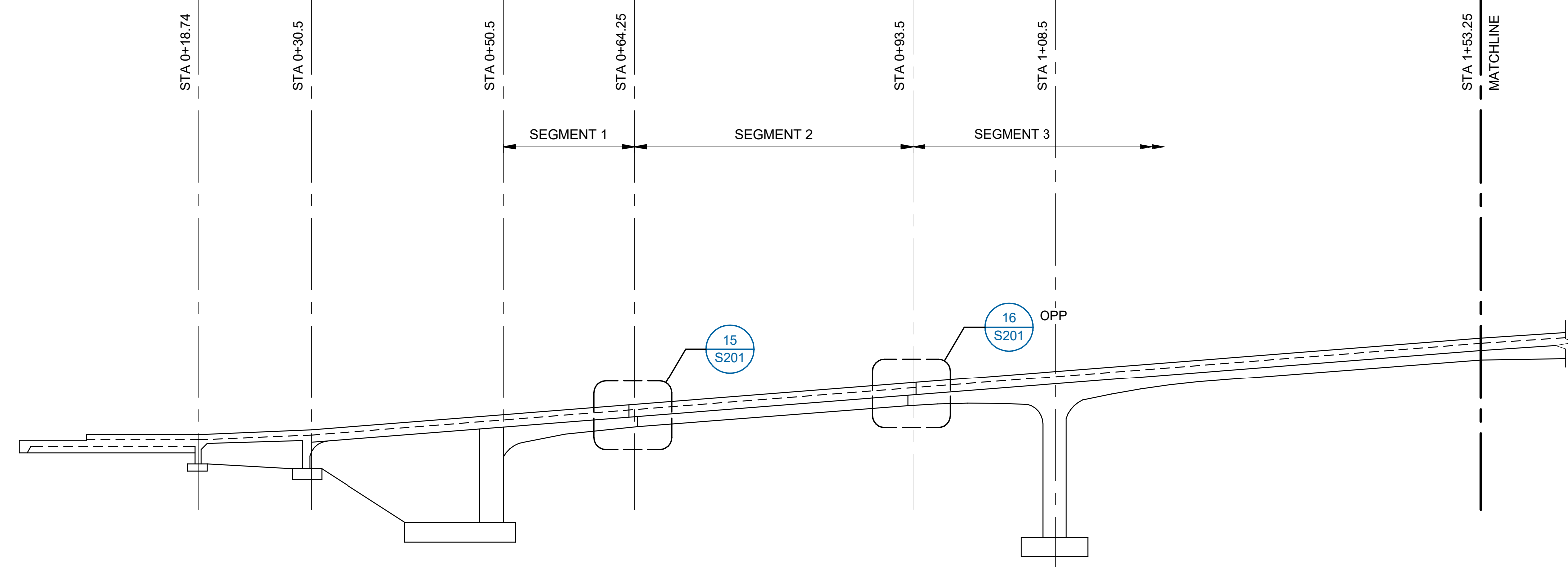
## STRUCTURAL NOTES AND CONSTRUCTION SEQUENCING

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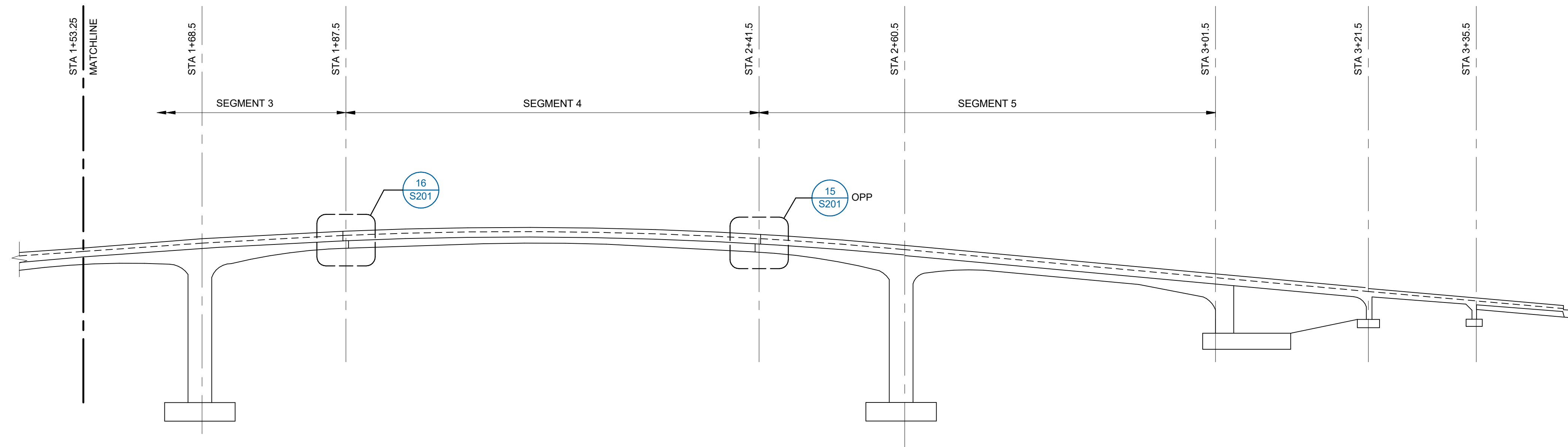
# S001



SEAL:



1 PEDESTRIAN WALKWAY ELEVATION - EAST END  
1" = 10'-0"



2 PEDESTRIAN WALKWAY ELEVATION - WEST END  
1" = 10'-0"

CLIENT: DES

CAPITOL WAY  
PEDESTRIAN  
WALKWAY  
REPAIR

OLYMPIA, WA

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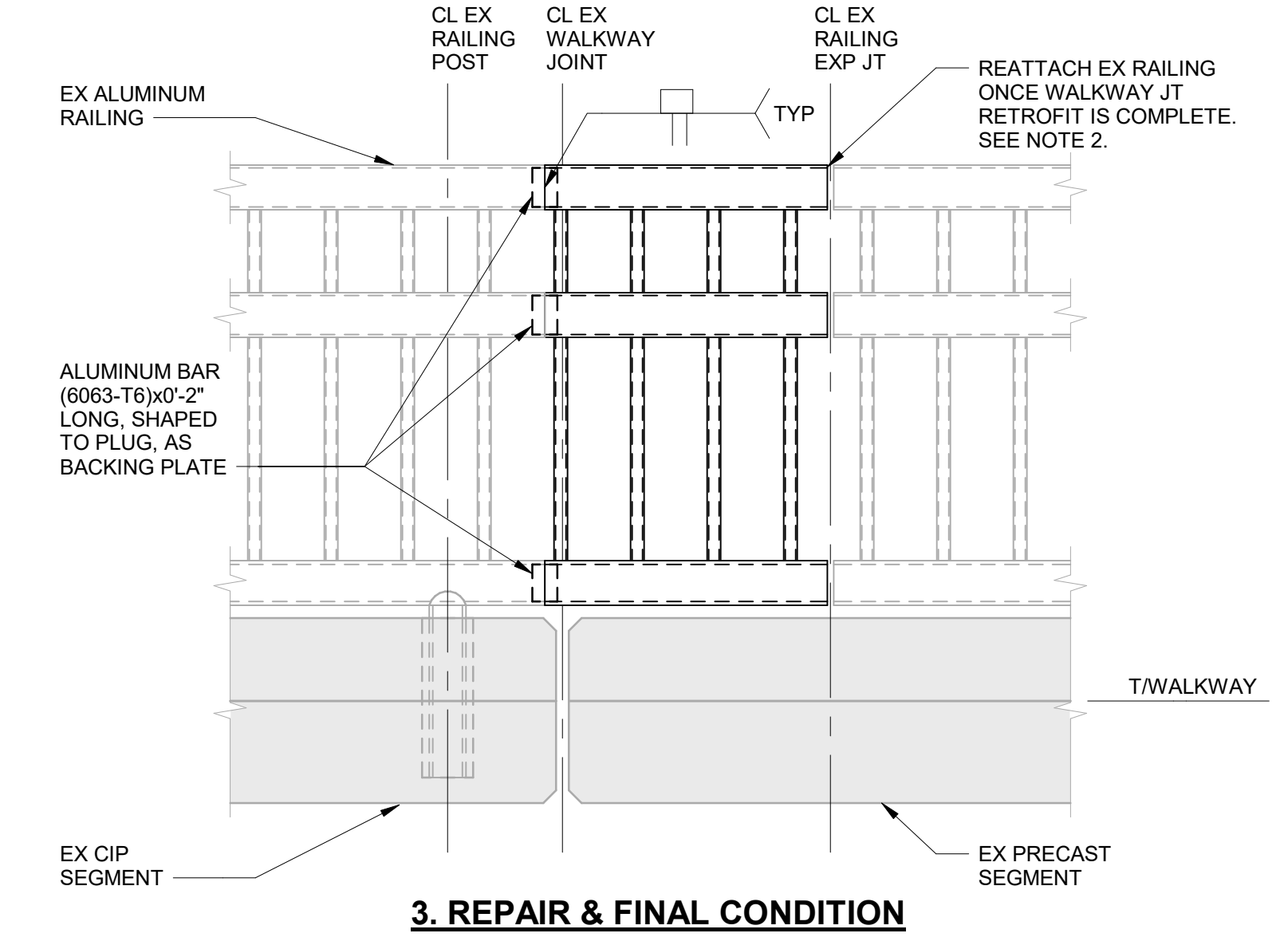
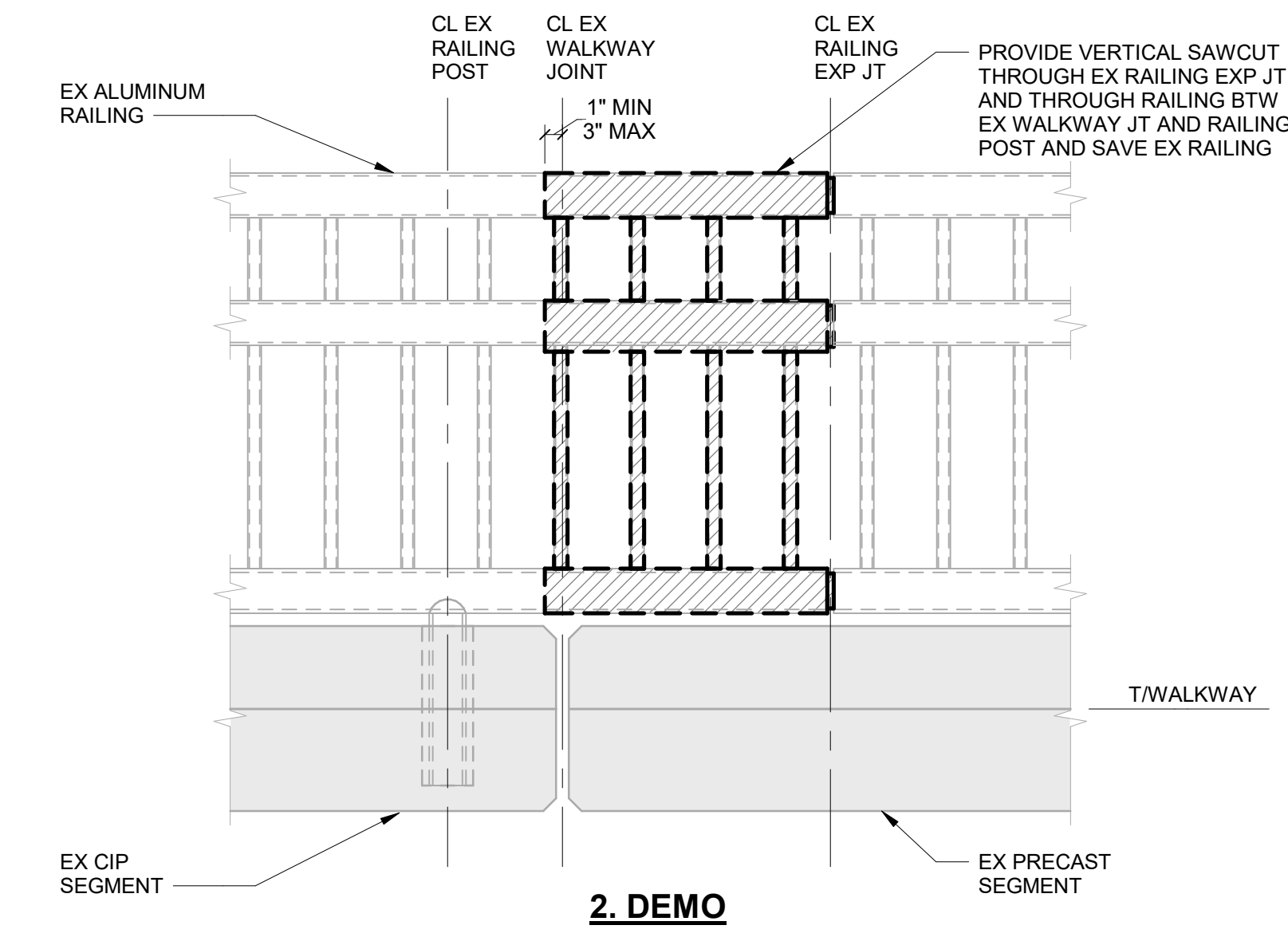
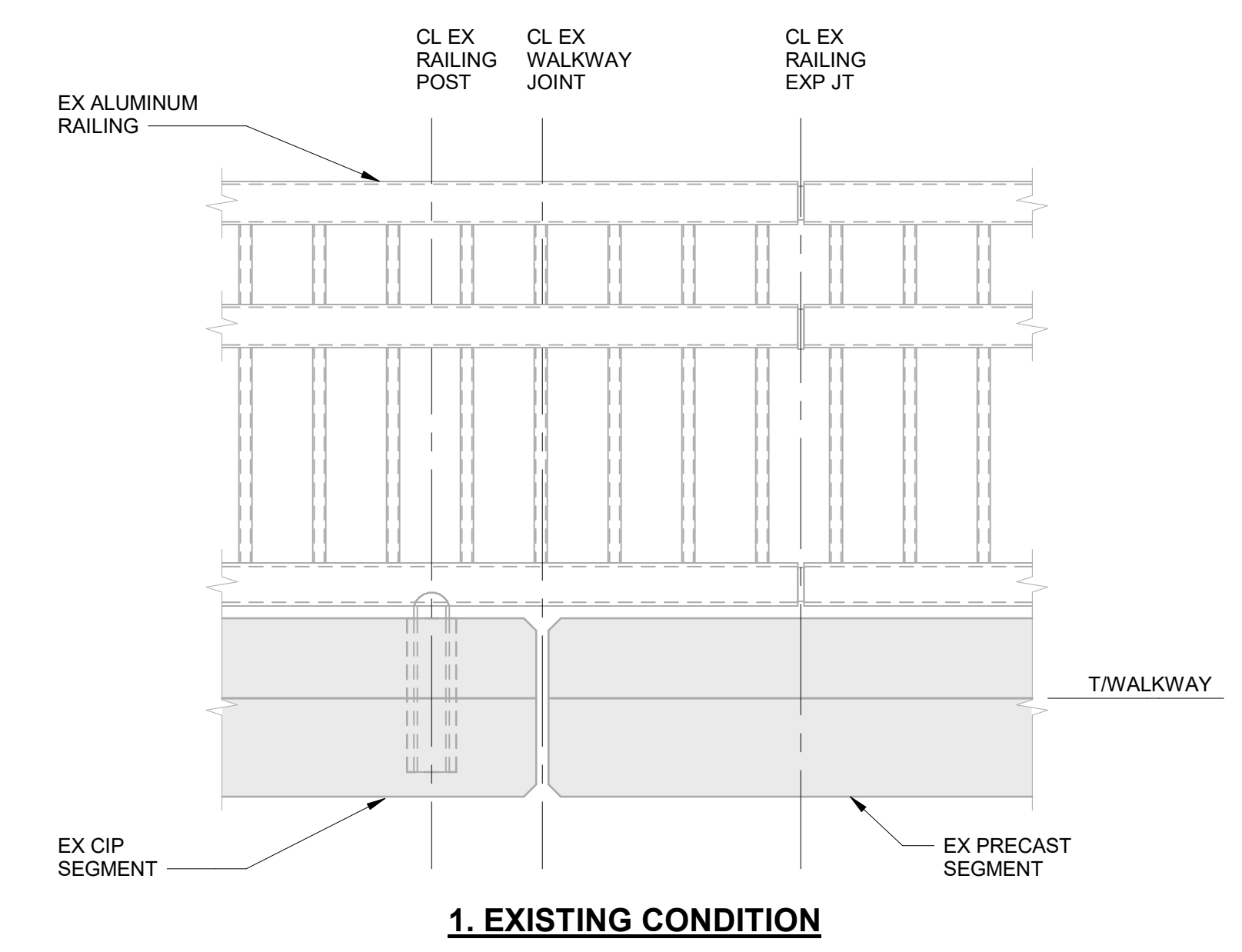
PEDESTRIAN  
WALKWAY  
ELEVATION

SHEET NUMBER:

S101



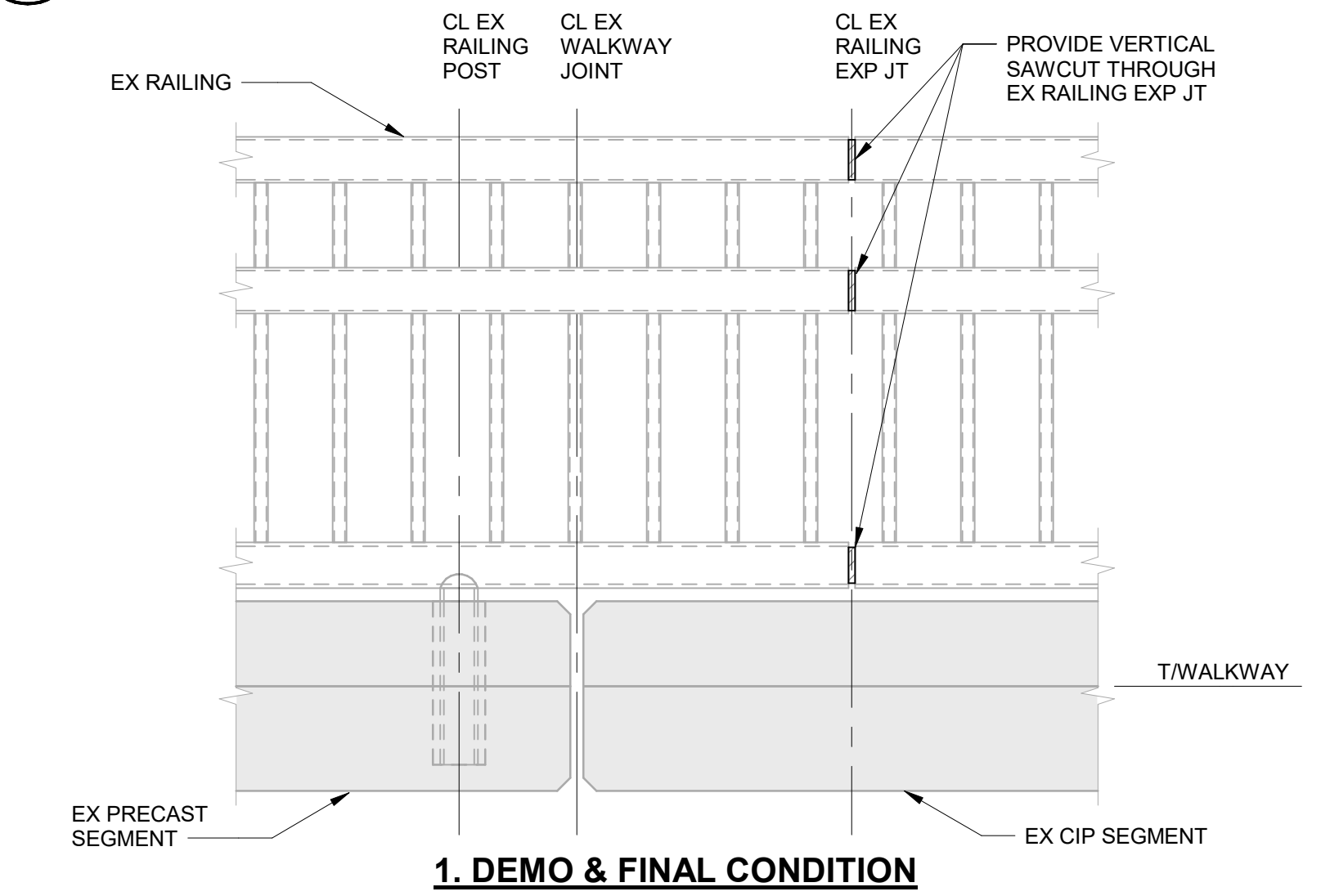
SEAL:



**NOTES:**  
1. WHERE RAILING CANTILEVERS FROM EX PRECAST SEGMENT OVER TO EX CIP SEGMENT OR WHEN RAILING EXPANSION JOINT OCCURS DIRECTLY OVER WALKWAY FIXED JOINT OR EXPANSION JOINT. SEE 13/S201 FOR RAILING EXPANSION JOINT MODIFICATION.  
2. VERIFY THAT GAP BETWEEN RAILING AT THE EXISTING RAILING EXPANSION JOINT IS AT LEAST 3/16" WIDE.

**RAILING EXPANSION JOINT MODIFICATION - OVERHANGING EX PRECAST SEGMENT**

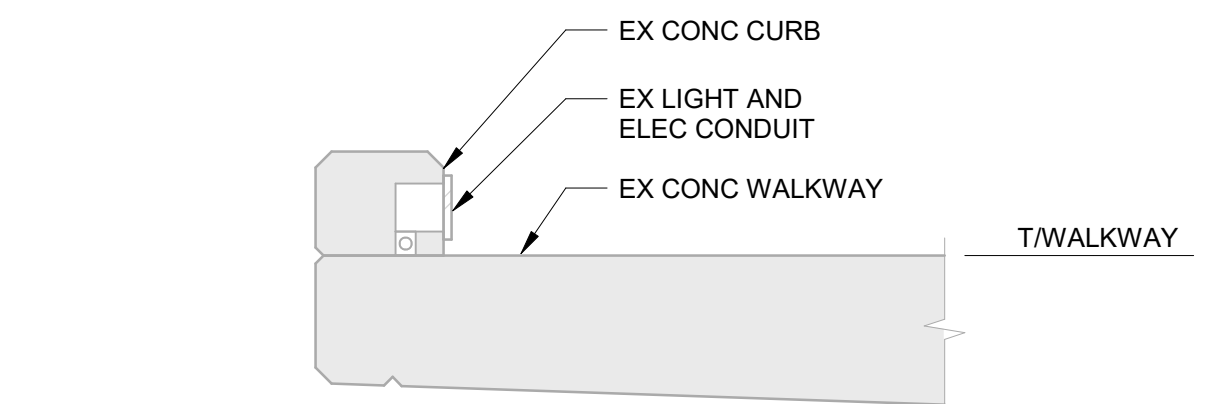
1" = 1'-0"



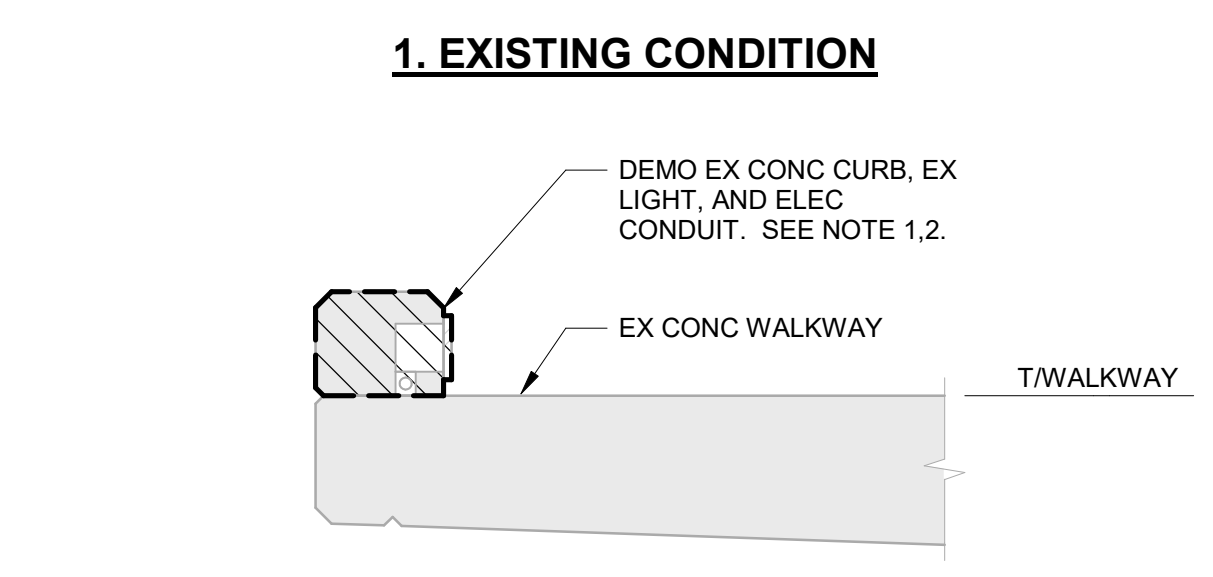
**NOTES:**  
1. AT SIM. RAILING EXPANSION JOINT OCCURS DIRECTLY OVER WALKWAY FIXED JOINT OR EXPANSION JOINT.  
2. WHERE RAILING CANTILEVERS FROM EX CIP SEGMENT OVER TO EX PRECAST SEGMENT. SEE 9/S201 FOR RAILING EXPANSION JOINT MODIFICATION.

**RAILING EXPANSION JOINT MODIFICATION - OVERHANGING EX CIP SEGMENT**

1" = 1'-0"



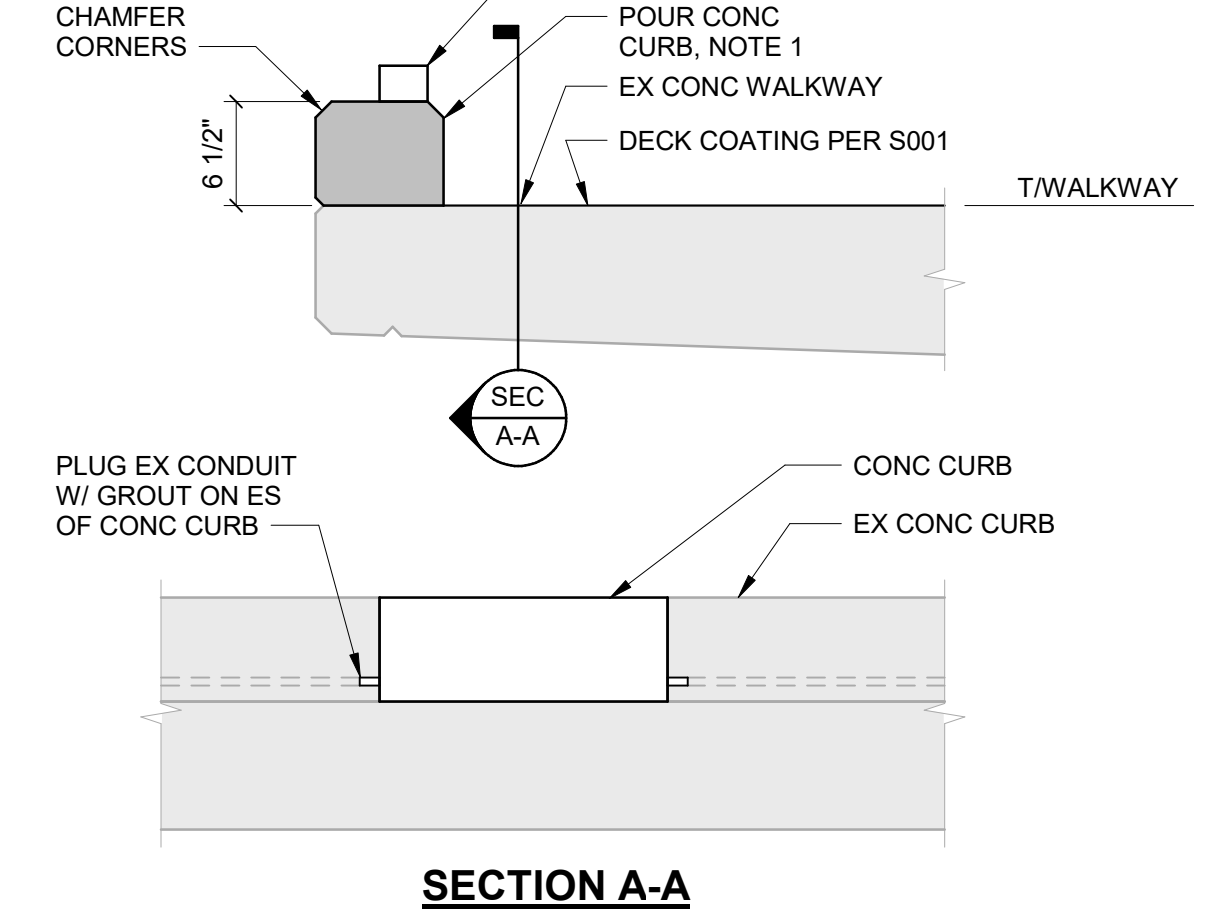
**NOTES:**  
1. EXISTING RAILING BEYOND NOT SHOWN FOR CLARITY.  
2. CONTRACTOR TO PROVIDE ROADWAY PROTECTION BELOW FROM FALL DEBRIS DURING EXISTING CONCRETE CURB DEMO.



**NOTES:**  
1. SPECIAL INSPECTOR TO DETERMINE EXTENT OF EXISTING CONCRETE CURB DEMO REQUIRED. WHERE EXISTING CONDUIT IS VISIBLE DUE TO CONCRETE SPALLING OCCURS OR WHERE CRACKS IN CONCRETE CURB OCCURS. DEMO EXISTING CONCRETE CURB, EXISTING LIGHT, AND EXISTING ELECTRICAL CONDUIT WITH VERTICAL SAWCUT 2" BEYOND SPALLING/CRACK. IF SAWCUT LINE OCCURS LESS THAN 6" FROM EXISTING CURB JOINT, DEMO TO JOINT. IF SAWCUT OCCURS WITHIN 6" BEYOND CENTERLINE OF EXISTING RAILING SLEEVE POCKET, DEMO CONCRETE 6" BEYOND CENTERLINE OF EXISTING RAILING SLEEVE POCKET.  
2. ANTICIPATED LENGTH OF CURB DEMO AND REPAIR IS APPROXIMATELY 100 LF TOTAL AT APPROXIMATELY 72 CURB JOINT LOCATIONS.  
3. EXISTING RAILING BEYOND NOT SHOWN FOR CLARITY.  
4. LEAVE EXISTING RAILING POST AS IS AND DEMO CURB AROUND RAILING WHERE OCCURS.

**SEQUENCING OF CONSTRUCTION FOR CONC CURB REPAIR**

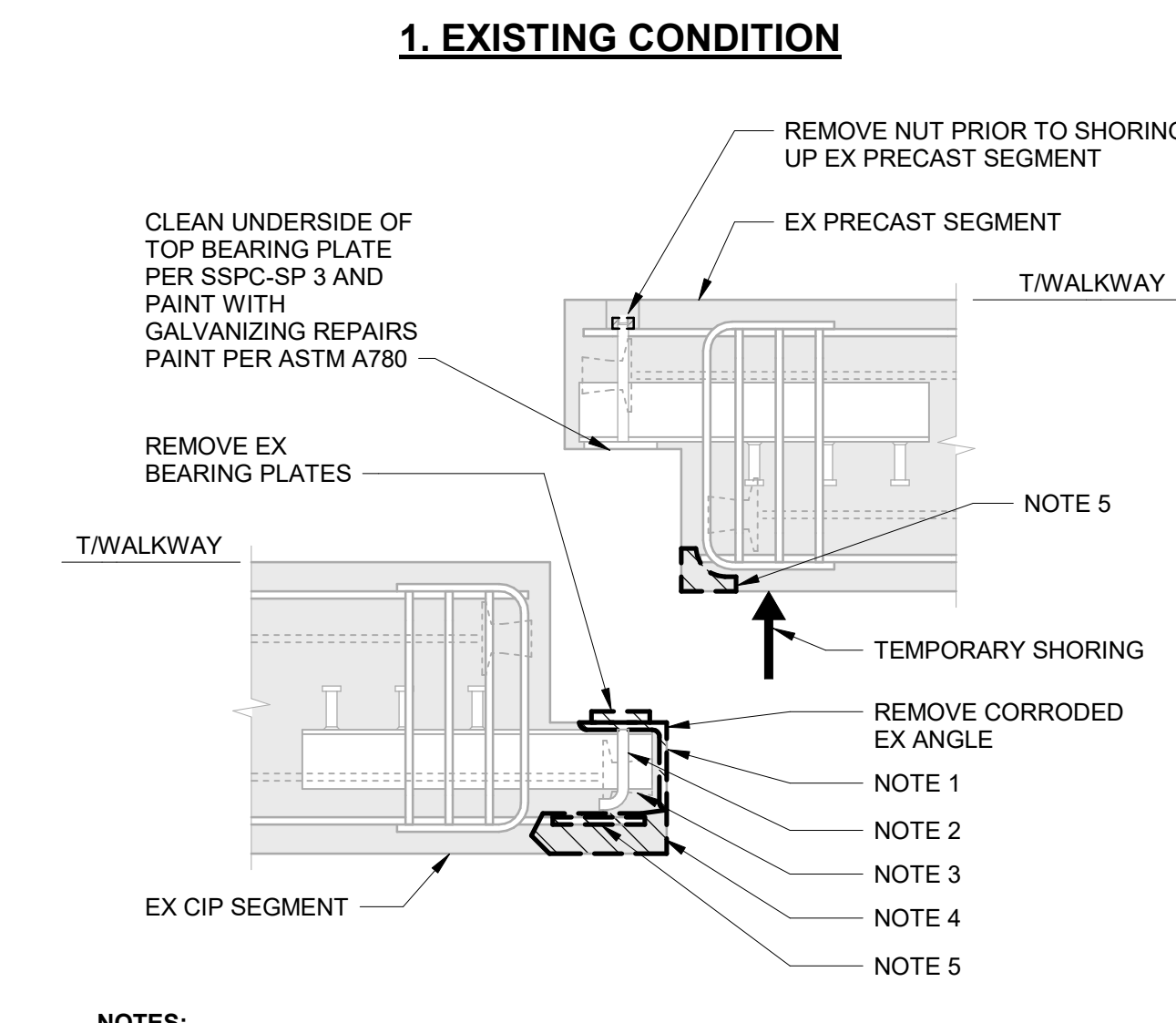
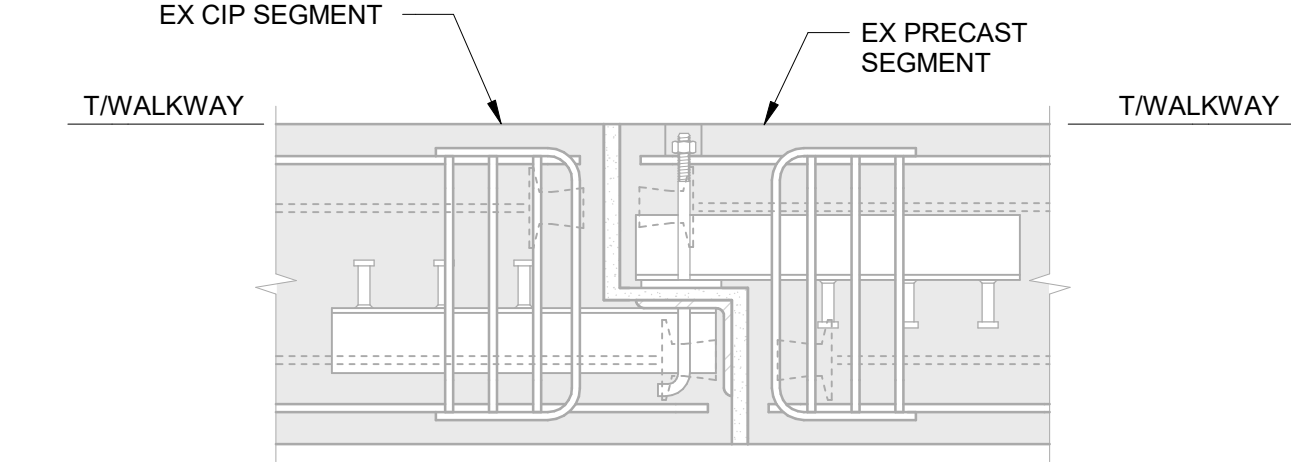
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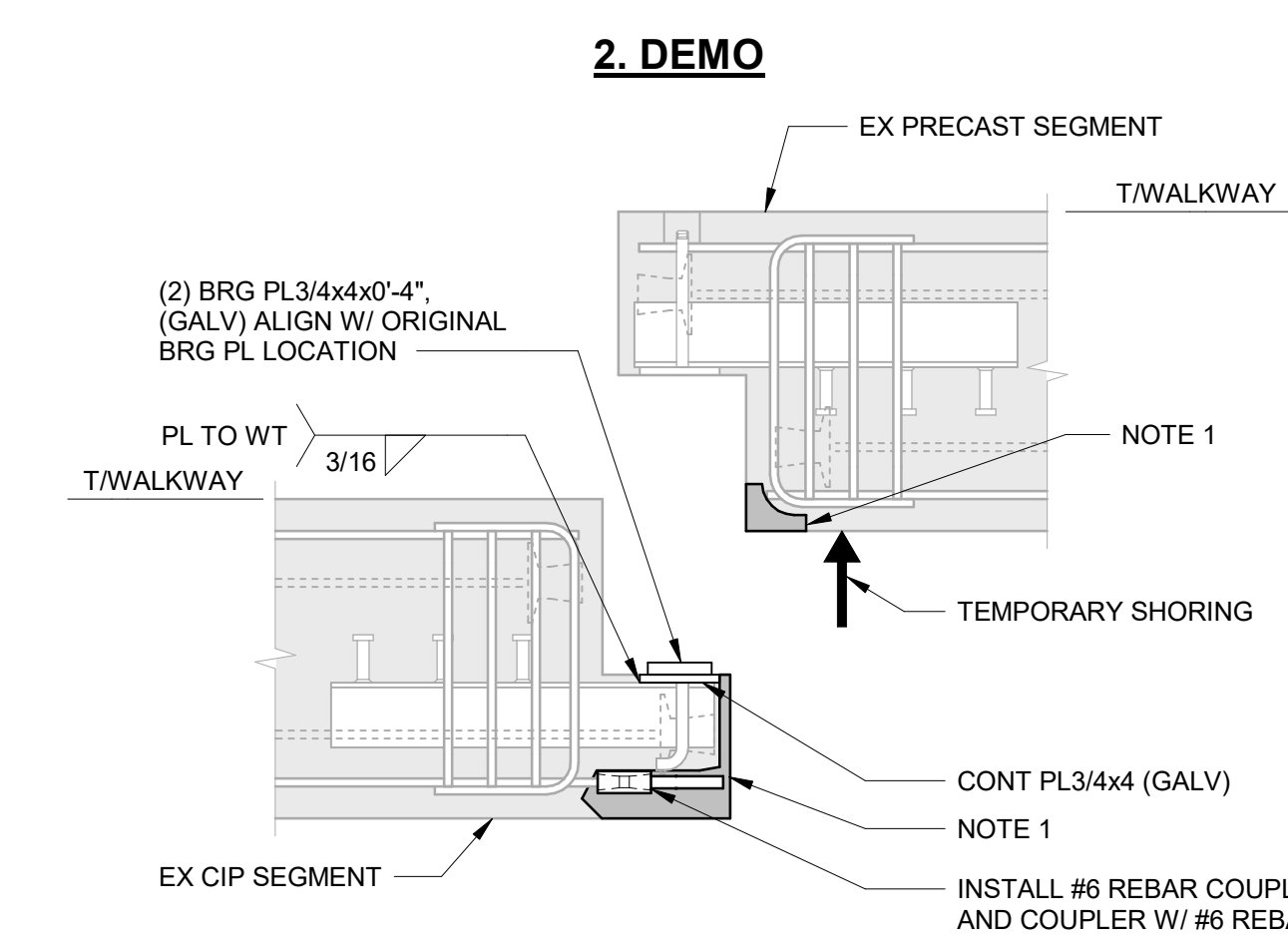
**NOTES:**  
1. CURB TO BE POURED AROUND EXISTING RAILING WHERE OCCURS. START AND STOP CONCRETE CURB ON EACH SIDE OF FIXED JOINT AND EXPANSION JOINT OF WALKWAY. PROVIDE ADDITIONAL 3/8" WIDE JOINTS IN CURB. CURB JOINT LOCATIONS TO MATCH ORIGINAL CURB JOINT LOCATION. PROVIDE 1" JOINT BETWEEN FIXED JOINT AND EXPANSION JOINT OF WALKWAY. PROVIDE PREMOLD JOINT FILLER AT ALL CURB JOINTS.

**SEQUENCING OF CONSTRUCTION AT FIXED JOINT**

1" = 1'-0"



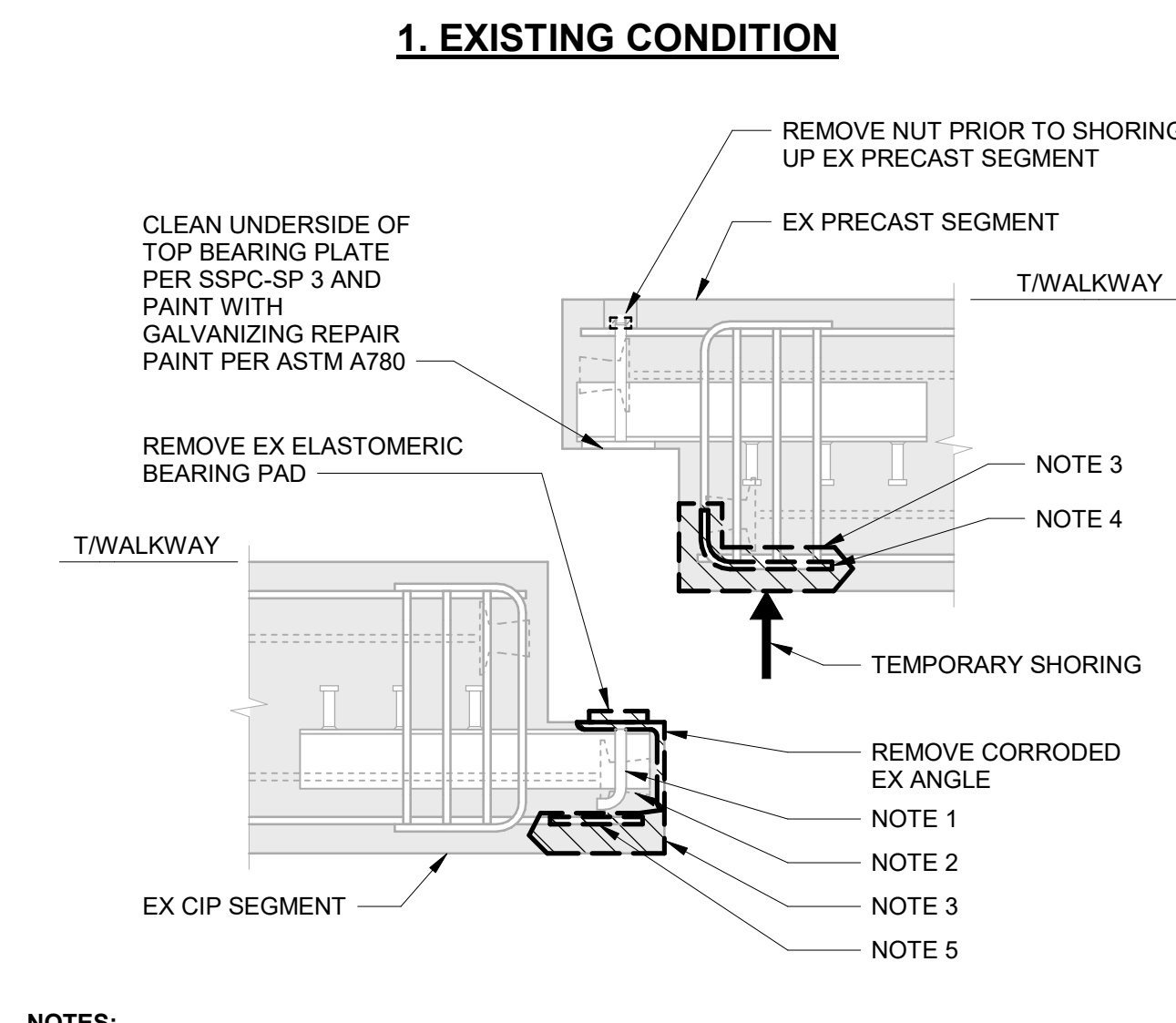
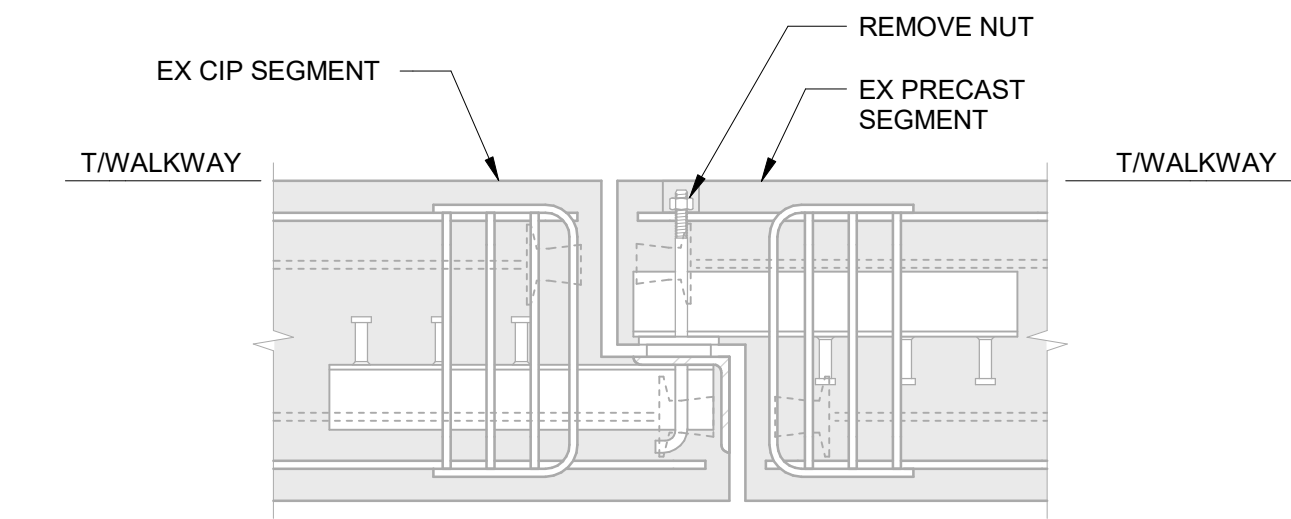
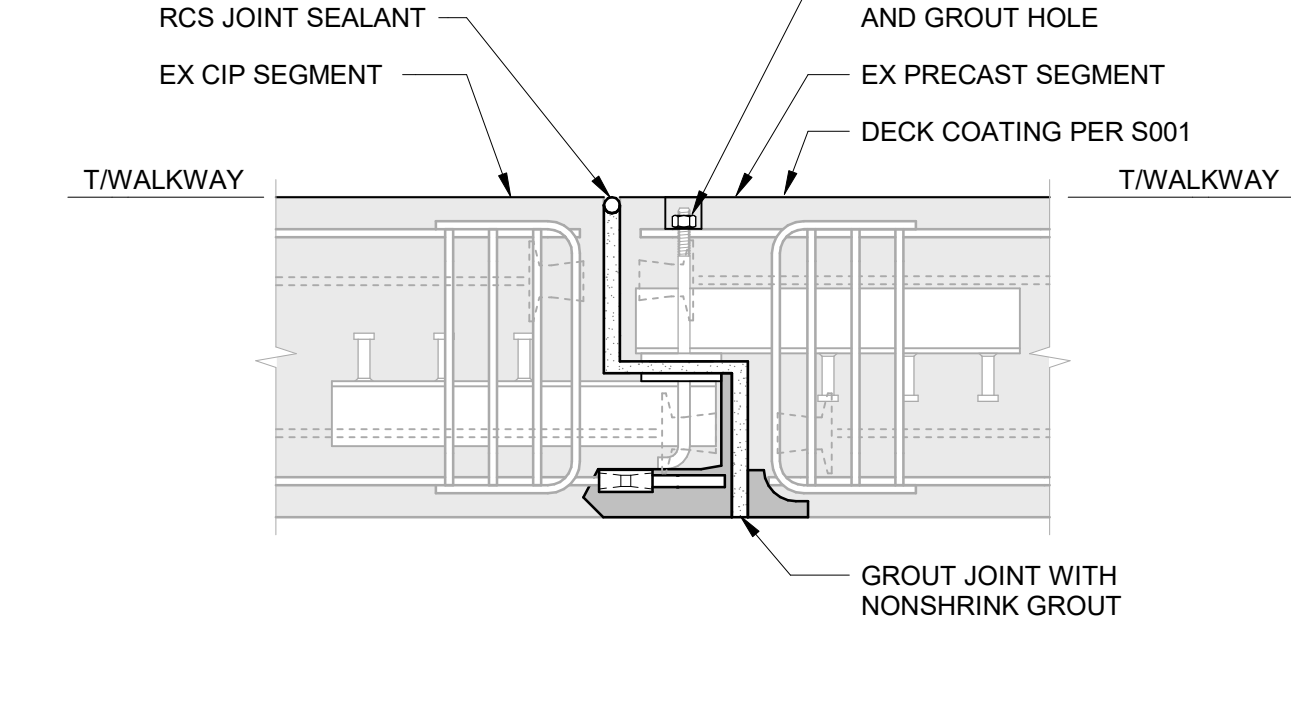
**NOTES:**  
1. REMOVE EXISTING GROUT FROM FIXED JOINT.  
2. EXISTING STUD IS ATTACHED TO EXISTING ANGLE. SPECIAL INSPECTOR TO EVALUATE THE EXTENT OF CORROSION AND REQUIRED DEMOLITION. REMOVE CORRODED PORTION OF STUD. OTHERWISE, STUD TO REMAIN.  
3. SPECIAL INSPECTOR TO DETERMINE WHETHER BONDED PT TENDONS ARE DAMAGED FROM CORROSION. IF BONDED PT TENDONS ARE DAMAGED, NOTIFY STRUCTURAL ENGINEER OF EXTENT OF DAMAGE PRIOR TO CUTTING BACK DAMAGED TENDON AND REPLACING WITH GROUT.  
4. REMOVE LOOSE AND DELAMINATED CONCRETE TO CIP SEGMENT PER "CONCRETE SURFACE REPAIR" SECTION OF GENERAL NOTES. BE AWARE OF PT HEADS AND DO NOT REMOVE CONCRETE BEHIND PT HEADS. AT CORRODED REBAR, CHIP BACK CONCRETE TO ALLOW FOR REBAR COUPLER INSTALLATION. DO NOT CUT EXISTING PT TENDONS. DO NOT CUT EXISTING REBAR BEYOND NOTE 5.  
5. SPECIAL INSPECTOR TO EVALUATE THE EXTENT OF STEEL CORROSION AND REQUIRED DEMOLITION. CUT BACK CORRODED EXISTING REBAR 1/2" BEYOND VISIBLE CORROSION.



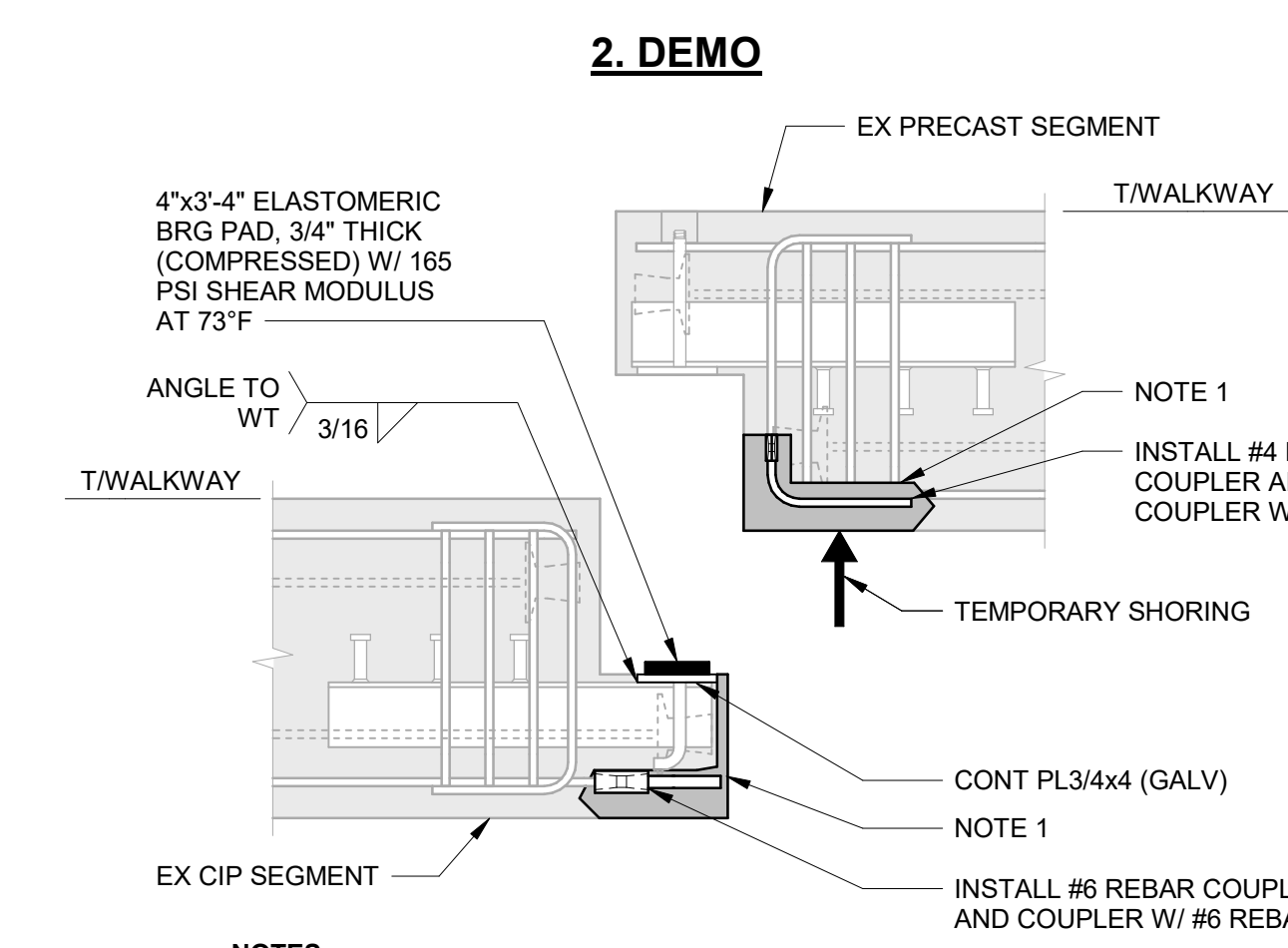
**NOTES:**  
1. PREPARE SURFACE PER "CONCRETE SURFACE REPAIR" SECTION OF GENERAL NOTES AND APPLY MORTAR TO REPLACE REMOVED CONCRETE.

**SEQUENCING OF CONSTRUCTION AT EXPANSION JOINT**

1" = 1'-0"



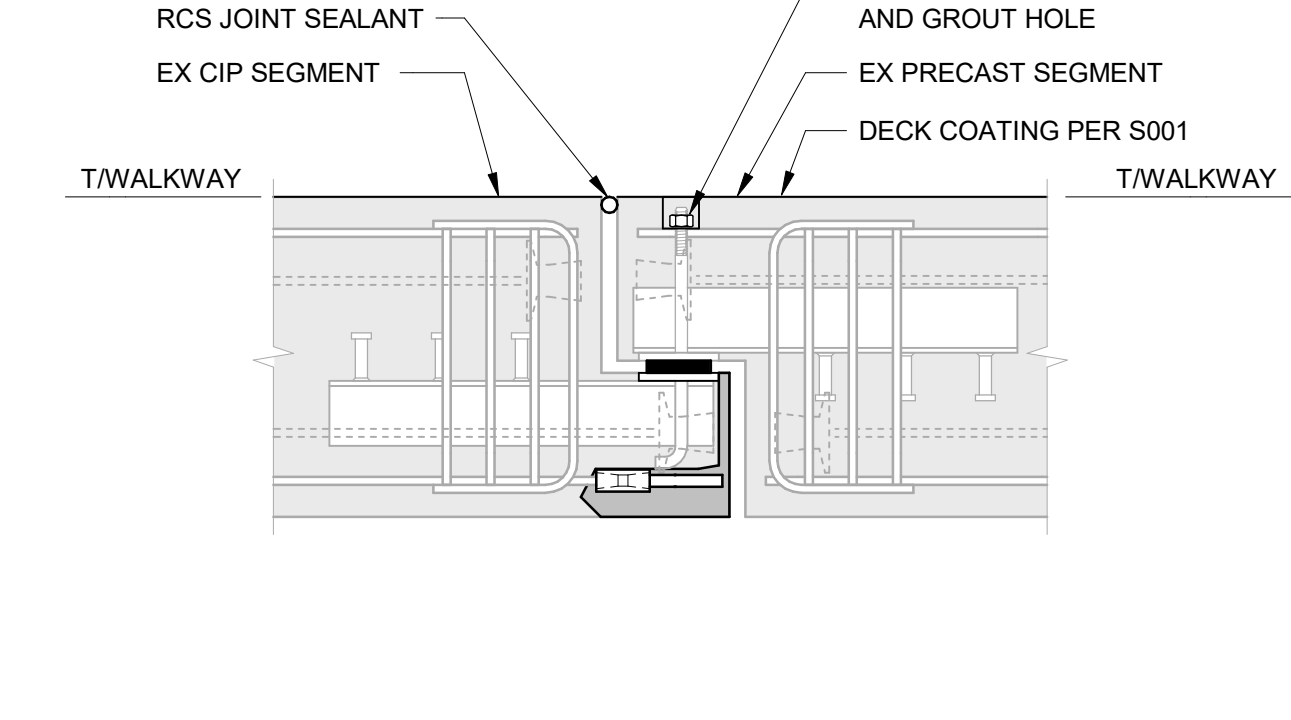
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**NOTES:**  
1. PREPARE SURFACE PER "CONCRETE SURFACE REPAIR" SECTION OF GENERAL NOTES AND APPLY MORTAR TO REPLACE REMOVED CONCRETE.

**SEQUENCING OF CONSTRUCTION AT EXPANSION JOINT**

1" = 1'-0"



CLIENT: DES

**CAPITOL WAY PEDESTRIAN WALKWAY REPAIR**  
OLYMPIA, WA

WA STATE DES PROJECT NUMBER:  
2026-195 G (1-1)

SUBMITTAL:

ISSUE DATE: --/--

REVISIONS:

MARK	DATE	DESCRIPTION

SHEET NAME:

**STRUCTURAL DETAILS**

SHEET NUMBER:

**S201**